

2305/303  
2307/303  
2308/303  
STRUCTURES  
Oct./Nov. 2009  
Time: 3 hours

THE KENYA NATIONAL EXAMINATIONS COUNCIL

**DIPLOMA IN BUILDING  
DIPLOMA IN CIVIL ENGINEERING  
DIPLOMA IN HIGHWAY ENGINEERING**

STRUCTURES

3 hours

**INSTRUCTIONS TO CANDIDATES**

*You should have the following for this examination:*

*Answer booklet*

*Mathematical tables/Pocket calculator*

*Drawing instruments.*

*Answer any FIVE of the EIGHT questions in this paper.*

*ALL questions carry equal marks.*

*Maximum marks for each part of a question are as shown.*

*Relevant design tables are provided.*

This paper consists of 11 printed pages.

Candidates should check the question paper to ascertain that all the pages are printed as indicated and that no questions are missing.

1. (a) Using the method of sections, determine the magnitude and nature of forces in the members of the frame shown in figure 1. (7 marks)

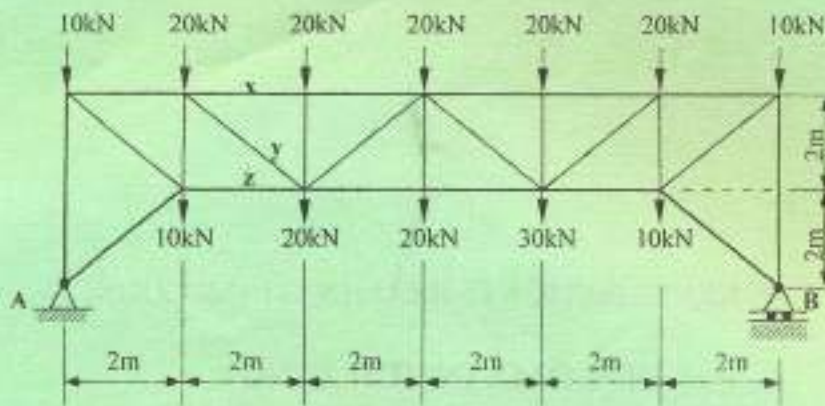


Fig 1

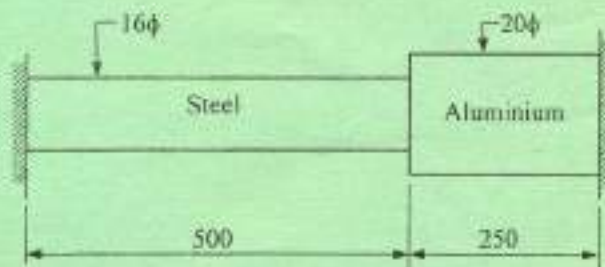
- (b) A composite bar is made up of two materials as shown in figure 2. If the bars are stress free at  $40^{\circ}\text{C}$ , determine the stresses developed in the bars when temperature drops to  $20^{\circ}\text{C}$ , when;
- the supports are unyielding
  - the supports come nearer to each other by  $0.12\text{mm}$ .

Given:

Aluminium:  $E_a = 70\text{kN/mm}^2$   
 $\alpha_a = 23.4 \times 10^{-6}$  per  $^{\circ}\text{C}$

Steel:  $E_s = 210\text{kN/mm}^2$   
 $\alpha_s = 11.7 \times 10^{-6}$  per  $^{\circ}\text{C}$

(13 marks)



Note: Dimensions in mm

Fig 2

2. Figure 3 shows a beam ABCDE built in at A and supported on rollers at B, C and D, with DE being an overhang. The values of moment of inertia of the section over each of these lengths are  $3I$ ,  $2I$ ,  $I$  and  $I$  respectively, the loading being as shown. Analyse the beam using the three moments theorem, and hence draw the bending moment diagram, indicating all the critical values. (20 marks)

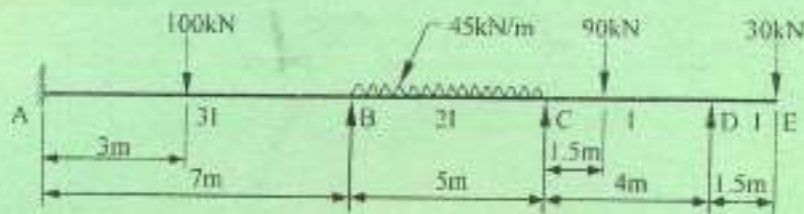


Fig 3

3. Using the method of moment distribution, analyse the frame shown in figure 4 and plot the bending moment diagram indicating all the critical values. (20 marks)

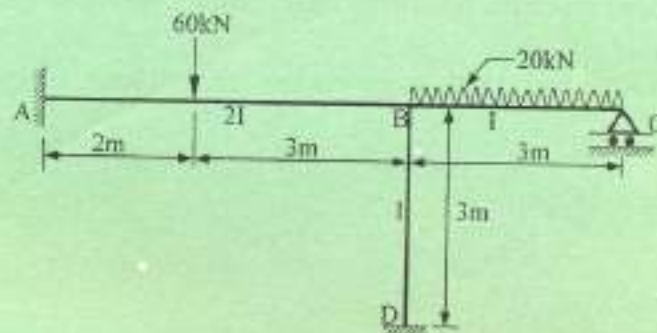


Fig 4

4. (a) A horizontal simply supported girder 14m long is of uniform section, and carries two point loads as shown in figure 5. Using Macaulay's method, determine the deflection under each point load. Take  $I = 1.6 \times 10^9 \text{mm}^4$ , and  $E = 210 \text{kN/mm}^2$ . (9 marks)

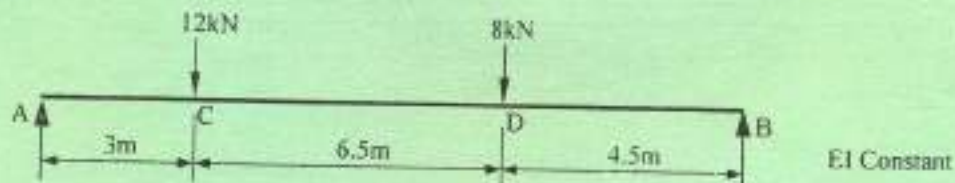


Fig 5

- (b) Figure 6 shows a loaded simply supported beam and its cross-section. Draw the shear stress distribution diagram indicating the critical values for the maximum shear force.

(11 marks)

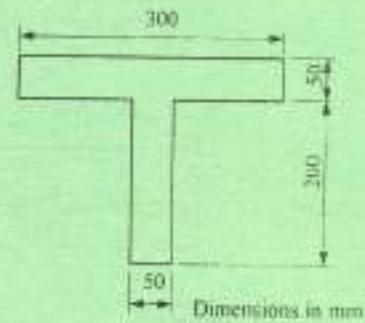
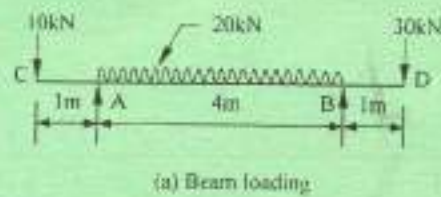
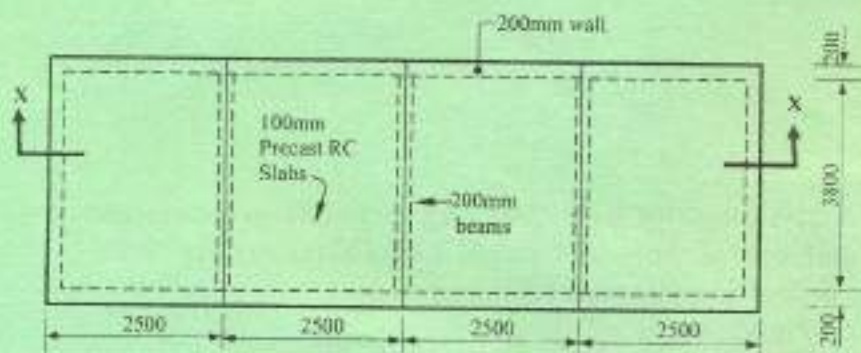


Fig 6

5. Figure 7 shows the plan and section through a precast concrete floor. Using the load factor method, design the beam given the following information:

- Concrete mix 1:24
- Beams are 200mm wide and simply supported on 200mm load bearing walls.
- live load =  $3 \text{ kN/m}^2$
- Finishes =  $1 \text{ kN/m}^2$
- Density of concrete =  $2400 \text{ kg/m}^3$
- $P_{st} = 230 \text{ N/mm}^2$ .

(20 marks)



Note: All dimensions in mm

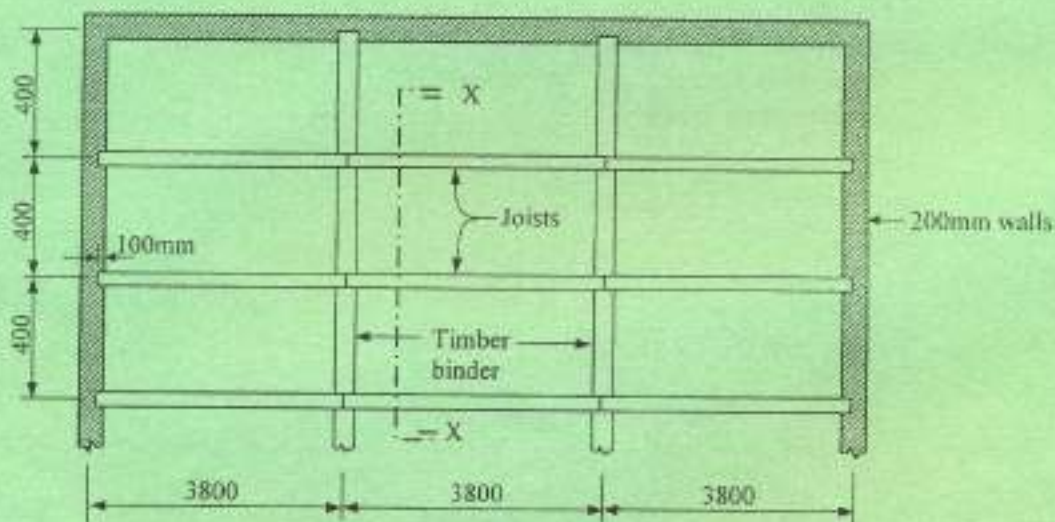
Fig 7

6. (a) A square column of size 300 X 300mm is to transmit an axial load of 700kN to its base. The column height centre to centre of floors is 3m, and is properly restrained at both ends in position and direction. Design the column and its base given the following information:
- concrete mix 1:1½:3
  - $P_{st} = 140\text{N/mm}^2$
  - $P_{cc} = 6.5\text{N/mm}^2$
  - $P_{sc} = 125\text{N/mm}^2$
  - $m = 15$
  - bearing capacity of soil =  $250\text{kN/m}^2$
  - Assume any other relevant information. (18 marks)
- (b) Detail the reinforcement for the column and its base as designed in (a). (2 marks)
7. (a) (i) Differentiate between 'basic stress' and 'green stress' as applied to timber.
- (ii) Explain each of the following in stress grading of timber:
- visual stress grading
  - machine stress grading. (6 marks)

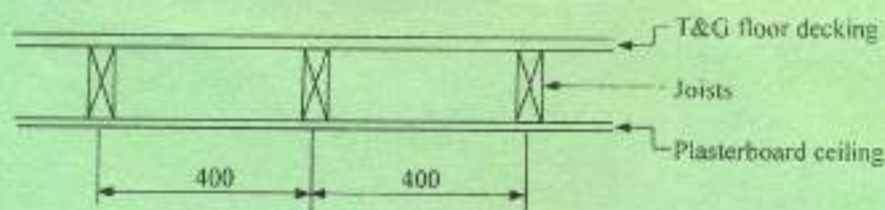
(b) Figure 8 shows the plan and section through a timber floor for a domestic dwelling. Design the timber joists for strength class SC2 given the following information:

- Joists are spaced at 400mm centres
- Joists have an effective span of 3.8m
- Self weight of T & G boards =  $0.1 \text{ kN/m}^2$
- Self weight of plasterboard ceiling =  $0.2 \text{ kN/m}^2$
- Imposed loading on floor =  $1.5 \text{ kN/m}^2$
- Depth of joist limited to 200mm
- Density of timber of SC2 class =  $540 \text{ kg/m}^3$
- Modification factor  $K3$  is as given in Table 1
- Modification factor for load sharing systems,  $K8 = 1.1$
- Depth factor,  $K7 = \left(\frac{300}{h}\right)^{0.11}$ , where  $h$  = depth of beam
- Maximum depth to breadth ratio is as given in Table 2
- Grade stresses and modulus of elasticity for SC2 class is as given in Table 3.

(14 marks)



PLAN



Note: All dimensions in mm

SECTION X-X

Fig 8

8. (a) Figure 9 shows the roof plan of a proposed hall. The roof consists of 125mm thick reinforced concrete slab support on universal beams. Check the adequacy of 533 X 165mm X 73 kg/m universal beams in grade 43 steel for the roof given the following information:

- spacing of universal beams = 2.5m centres
- roof finish together with waterproof layer of thickness 75mm is of average specific weight  $20\text{kN/m}^3$
- Live load on roof finish =  $0.75\text{kN/m}^2$ .
- Density of reinforced concrete =  $2400\text{kg/m}^3$ .
- $E = 210\text{kN/mm}^2$
- $f_b = 165\text{N/mm}^2$
- $P_q = 100\text{N/mm}^2$
- Assume any other relevant information.

(12 marks)

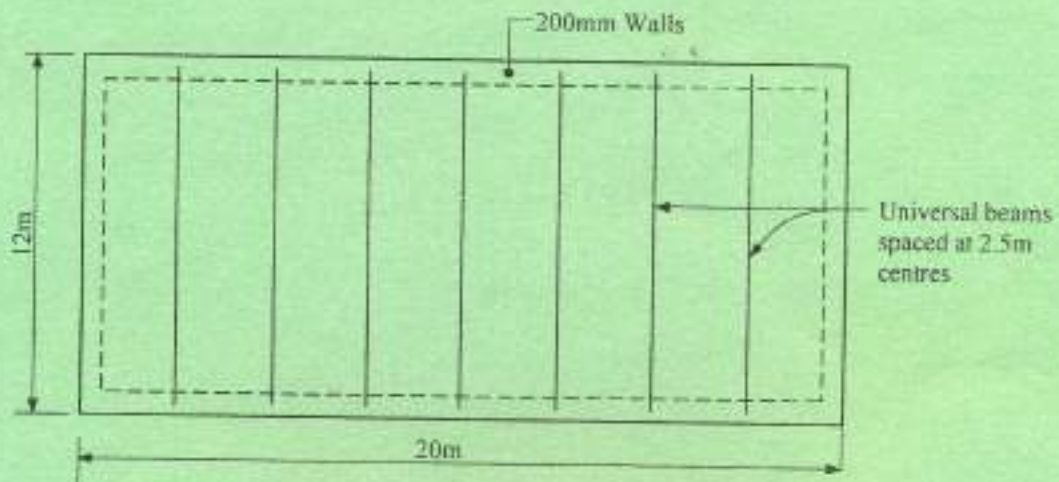


Fig 9

- (b) Figure 10 shows a proposed bolted connection. Determine the safe load  $P$ .  
Take  $f_t = 95\text{N/mm}^2$ ,  $f_c = 155\text{N/mm}^2$  and  $f_{br} = 300\text{N/mm}^2$

(8 marks)

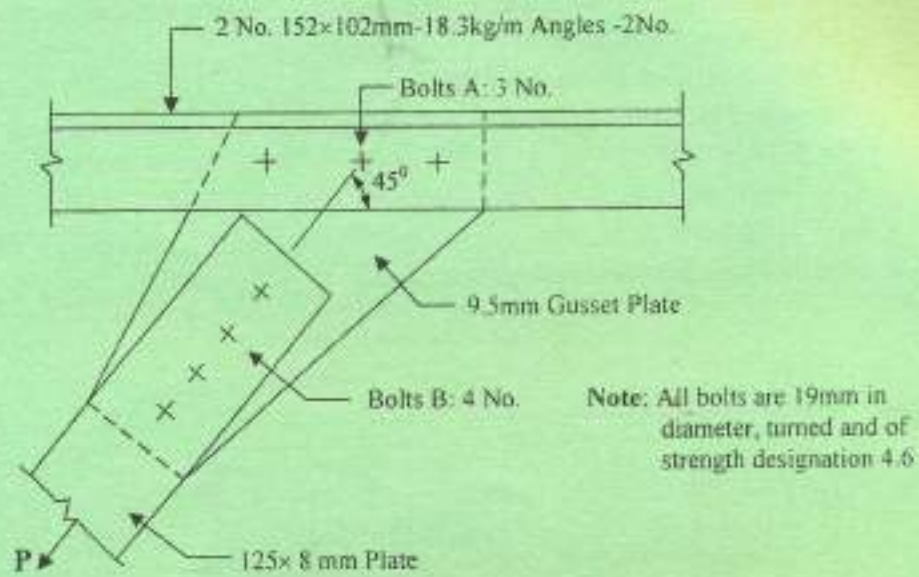


Fig 10



Duration of loading	Value of $K_3$
Long term (e.g. dead + permanent imposed)	1.00
Medium term (e.g. dead + snow, dead + temporary imposed)	1.25
Short term (e.g. dead + imposed + wind, dead + imposed + snow + wind)	1.50
Very short term (e.g. dead + imposed + wind)	1.75

**Table 2:** Depth factor,  $K_7$  (BS 5268)

1. $K_7 = 1.17$ for solid beams having a depth $< 72\text{mm}$
2. $K_7 = (300/h)^{0.11}$ for solid beams with $72\text{mm} < h < 300\text{mm}$
3. $K_7 = 0.81(h^2 + 92300)/(h^3 + 56800)$ for solid beams with $h < 300\text{mm}$

**Table 3:** Grade stresses, modulus of elasticity and density for strength class SC2 for the dry exposure condition (Table 9, BS 5268)

Strength Class	Bending parallel to grain ( $\text{Nmm}^{-2}$ )	Tension parallel to grain ( $\text{Nmm}^{-2}$ )	Compression parallel to grain ( $\text{Nmm}^{-2}$ )	Compression perpendicular to grain*		Shear parallel to grain ( $\text{Nmm}^{-2}$ )	Modulus of elasticity		Approximate Density ( $\text{kgm}^{-3}$ )
				( $\text{Nmm}^{-2}$ )	( $\text{Nmm}^{-2}$ )		( $\text{Nmm}^{-2}$ )	( $\text{Nmm}^{-2}$ )	
SC1	2.8	2.2	3.5	2.1	1.2	0.46	6800	4500	540
SC2	4.1	2.5	5.3	2.1	1.6	0.66	8000	5000	540
SC3	5.3	3.2	6.8	2.2	1.7	0.67	8800	5800	540
SC4	7.5	4.5	7.9	2.4	1.9	0.71	9900	6600	590
SC5	10.0	6.0	8.7	2.8	2.4	1.00	10700	7100	590/760
SC6	12.5	7.5	12.5	3.8	2.8	1.50	14100	11800	840
SC7	15.0	9.0	14.5	4.4	3.3	1.75	16200	13600	960
SC8	17.5	10.5	16.5	5.2	3.9	2.00	18700	15600	1080
SC9	20.5	12.3	19.5	6.1	4.6	2.25	21600	18000	1200

\* When the specification specifically prohibits wane at bearing areas, the higher values of compression perpendicular to the grain stress may be used; otherwise the lower values apply.

Table 4: Reinforcement-bar areas ( $\text{mm}^2$ ) per metre width for various bar spacings.

Bar Diameter (mm)	Bar spacing (mm)									
	75	100	125	150	175	200	225	250	275	300
6	377	283	226	189	162	142	126	113	103	94
8	671	503	402	335	287	252	223	201	183	168
10	1047	785	628	523	449	393	349	314	286	262
12	1508	1131	905	754	646	566	503	452	411	377
16	2681	2011	1608	1340	1149	1005	894	804	731	670
20	4189	3142	2513	2094	1795	1571	1396	1257	1142	1047
25	6545	4909	3927	3272	2805	2454	2182	1963	1785	1636
32	-	8042	6434	5362	4596	4021	3574	3217	2925	2681
40	-	-	10050	8378	7181	6283	5585	5027	4570	4189

Areas of group of reinforcement bars ( $\text{mm}^2$ )

Bar Diameter (mm)	Number of bars									
	1	2	3	4	5	6	7	8	9	10
6	28	57	85	113	141	170	198	226	254	283
8	50	101	151	201	251	302	352	402	452	503
10	79	157	236	314	393	471	550	628	707	785
12	113	226	339	452	565	679	792	905	1017	1131
16	201	402	603	804	1005	1206	1407	1608	1809	2011
20	314	628	942	1257	1571	1885	2199	2513	2827	3142
25	491	982	1473	1963	2454	2945	3436	3927	4418	4909
32	804	1608	2412	3216	4021	4825	5629	6433	7237	8042
40	1256	2513	3769	5026	6283	7539	8796	10050	11310	12570

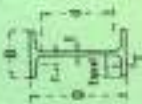
# UNIVERSAL BEAMS

DIMENSIONS AND PROPERTIES



# UNIVERSAL BEAMS

DIMENSIONS AND PROPERTIES



Serial No	Section Size	Moment of Inertia			Radius of Gyration			Static Modulus			Ratio $\frac{D}{T}$
		About X-X		About Y-Y	Axis X-X	Axis Y-Y	Axis X-X	Axis Y-Y	Axis X-X	Axis Y-Y	
		cm <sup>4</sup>	in <sup>4</sup>	cm <sup>4</sup>	cm	cm	cm <sup>3</sup>	cm <sup>3</sup>	cm <sup>3</sup>	in <sup>3</sup>	
814 x 419		717325	839177	42481	38.1	8.27	15886	2021	25.2		
		623068	550835	56251	37.8	9.11	13801	1733	28.1		
814 x 309		603791	409903	14793	37.0	6.34	10174	951.3	23.0		
		435794	408504	12512	36.8	6.23	8490	819.2	32.9		
838 x 292		375111	350209	10425	36.3	6.05	8241	805.6	34.1		
		324716	300783	8632	36.6	6.81	7102	669.1	44.7		
762 x 267		333130	315168	10851	34.3	6.08	3971	705.9	31.9		
		279832	259225	8384	33.6	5.82	6852	675.0	30.7		
686 x 254		245412	230057	7111	33.1	5.64	5675	661.8	44.4		
		239454	211138	7699	30.9	5.94	6223	574.0	30.1		
610 x 305		204747	199241	6376	30.5	5.28	5374	478.1	35.3		
		168535	156213	5002	30.0	5.18	4471	317.1	43.1		
610 x 229		155106	155106	8228	28.0	5.20	4902	490.8	29.2		
		130016	137965	5391	27.8	4.80	4304	423.7	32.7		
610 x 176		135972	126156	4789	27.6	5.18	3975	377.5	30.0		
		117900	108590	3982	27.2	5.00	3472	315.5	41.8		
533 x 330		202252	192203	14973	28.1	7.02	6549	861.2	20.2		
		151312	140289	10571	25.8	6.81	4931	680.6	28.2		
533 x 210		134341	113223	6471	25.8	6.08	4075	559.9	30.8		
		111072	101889	4293	25.0	4.88	3610	399.0	27.8		
533 x 185		80468	86675	3075	24.9	4.80	2817	291.1	31.2		
		72580	79649	3184	24.6	4.70	2517	275.1	35.1		
457 x 181		75540	66132	2658	24.2	4.54	2509	233.6	40.7		
		63970	57230	1427	23.9	3.51	2194	160.0	40.2		
457 x 151		50779	50076	1203	23.1	3.39	1805	135.3	46.7		
		161862	121777	18064	22.8	7.72	5199	863.2	19.6		
457 x 101		125618	107882	14083	22.8	7.84	4857	545.6	21.0		
		105100	83647	12057	22.6	7.53	4691	530.3	24.2		
457 x 81		76078	66718	3208	22.1	4.54	3794	307.8	35.6		
		66610	60218	2766	21.9	4.46	3486	287.9	38.7		
457 x 61		61520	56571	2512	21.9	4.46	3263	269.2	30.8		
		55225	50940	2212	21.7	4.24	2972	237.5	36.2		
457 x 41		47363	43062	1836	21.3	4.18	1783	173.0	40.0		
		40414	36762	1027	20.8	3.32	1520	134.1	39.2		
457 x 21		35003	31144	863	20.5	3.21	1337	104.3	42.6		
		49663	40469	2216	19.1	4.31	1954	228.9	37.8		
457 x 11		40654	36313	1900	18.0	4.15	1767	142.6	30.2		
		37038	32869	1748	18.8	4.09	1610	131.6	32.2		
457 x 1		33324	28570	1547	18.7	4.04	1420	102.4	31.8		
		26327	22672	1328	18.3	3.99	1283	93.8	34.7		

Serial No	Section Size	Mass per metre	Depth of Section D	Width of Section B	Thickness		Root Radius r	Depth between Flanges d	Area of Section
					Web t	Flange T			
814 x 419		388	823.5	420.5	21.5	26.6	24.1	781.5	493.9
		342	811.4	416.5	19.4	32.0	24.1	781.5	430.3
814 x 309		288	825.0	307.9	18.6	32.0	19.1	819.2	260.5
		259	815.6	305.6	17.9	32.9	19.1	819.2	222.5
838 x 292		259	815.3	305.1	15.9	30.2	19.1	819.2	284.9
		241	803.0	303.4	15.2	30.2	19.1	819.2	258.1
762 x 267		228	830.9	293.8	16.1	29.9	17.9	755.4	288.4
		194	840.7	292.4	14.7	27.7	17.9	755.4	233.0
686 x 254		178	834.9	291.8	14.0	18.8	17.9	755.4	223.0
		197	769.6	268.0	15.6	25.4	16.0	685.2	250.6
610 x 305		133	762.0	266.7	14.3	21.8	16.0	685.2	220.2
		147	763.9	269.3	12.9	17.5	16.0	685.2	187.8
610 x 229		170	692.9	255.8	14.5	23.7	16.2	610.6	216.2
		162	687.6	254.5	13.2	21.0	16.2	610.6	192.0
610 x 176		140	683.5	253.7	12.4	19.0	16.2	610.6	178.4
		129	677.5	253.0	11.7	18.2	16.2	610.6	169.4
533 x 330		229	633.0	311.8	18.6	31.4	16.5	531.8	303.5
		179	617.5	307.0	14.1	23.0	16.5	531.8	237.7
533 x 210		149	609.5	304.8	11.9	19.7	16.5	531.8	199.9
		140	617.0	290.1	12.1	22.1	12.7	543.1	178.2
533 x 185		129	611.9	289.0	11.9	19.6	12.7	543.1	159.4
		113	607.2	286.2	11.3	17.9	12.7	543.1	144.3
533 x 151		101	602.2	277.6	10.8	14.8	12.7	543.1	129.0
		91	602.5	178.4	10.8	15.0	12.7	547.1	115.9
533 x 101		82	598.1	177.8	10.1	12.8	12.7	547.1	104.4
		212	545.1	333.6	16.7	37.8	18.5	450.1	289.6
533 x 81		189	539.5	331.7	14.9	35.0	18.5	450.1	241.2
		167	533.4	330.2	13.4	32.0	18.5	450.1	212.7
533 x 61		122	544.6	211.9	12.8	21.2	12.7	472.7	156.6
		109	538.5	210.1	11.0	18.8	12.7	472.7	138.4
533 x 41		101	535.1	209.3	10.2	17.4	12.7	472.7	117.6
		92	528.3	208.7	9.6	15.2	12.7	472.7	104.3
533 x 21		73	528.8	165.6	9.3	13.6	12.7	470.5	93.0
		66	524.8	165.1	8.8	11.6	12.7	470.5	83.6
533 x 1		96	487.4	192.8	11.4	18.6	10.2	404.4	125.2
		89	480.7	192.0	10.6	17.7	10.2	404.4	113.8
533 x 1		82	440.7	181.3	9.9	16.0	10.2	404.4	104.4
		74	437.2	180.5	9.1	14.8	10.2	404.4	94.9
533 x 1		67	433.6	180.5	8.5	12.7	10.2	404.4	85.4